

TITAN PLATE C CONCRETE

PLATE FOR SHEAR LOADS

VERSATILE

It can be used for continuous connection to the substructure of both CLT and light timber frame walls.

INNOVATIVE

Designed to be partially or completely fastened with nails or screws. Possibility of installation even in the presence of bedding grout.

CALCULATED AND CERTIFIED

CE marking according to EN 14545. Available in 2 versions. TCP300 with increased thickness optimised for CLT.



USA, Canada and more design values available online.



SERVICE CLASS



MATERIAL

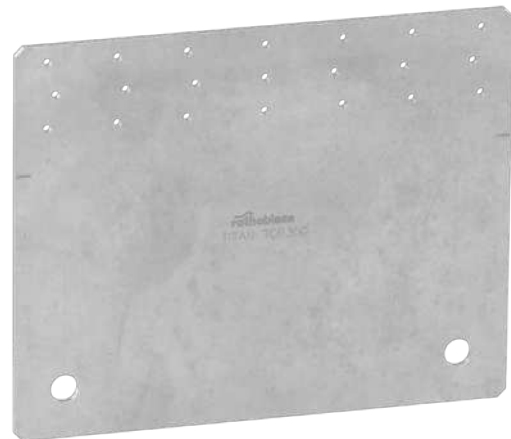
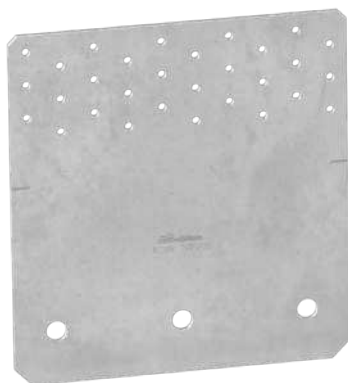
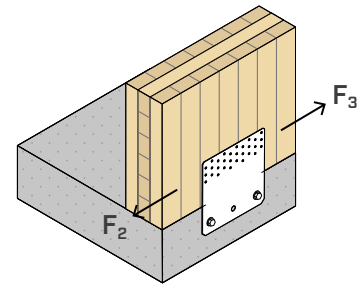
DX51D
Z275

TCP200: DX51D + Z275 carbon steel

S355
Fe/Zn12c

TCP300: S355 + Fe/Zn12c carbon steel

EXTERNAL LOADS



FIELDS OF USE

Shear joints for timber walls.
Timber-to-concrete or timber to-steel configurations.
Suitable for walls aligned to the concrete edge.

Can be applied to:

- solid timber and glulam
- timber frame
- CLT and LVL panels



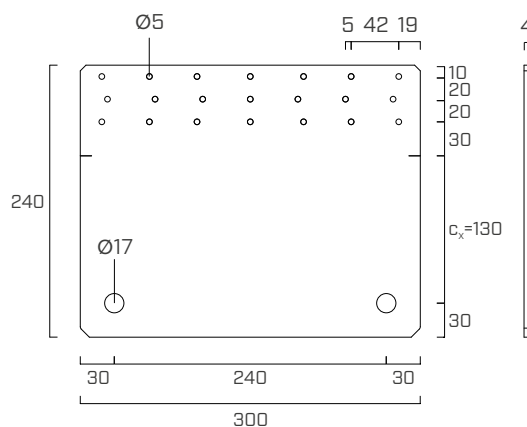
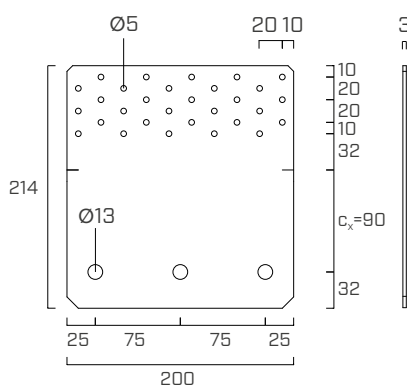
ADDED STOREYS

Ideal for making flat joints between concrete or masonry elements and CLT panels. Construction of continuous shear connections.

HYBRID STRUCTURES

Within hybrid timber-to-steel structures, it can be used for shear connections by simply aligning the edge of the timber with the edge of the steel element.

TCP300



A diagram showing a rectangular plate with a grid of points. A horizontal line is drawn across the plate, labeled a_1 on the right. The plate is divided into two regions by this line: a top region with a grid of points and a bottom region with a grid of points. A double-headed arrow is shown on the left side of the plate, indicating a horizontal dimension.

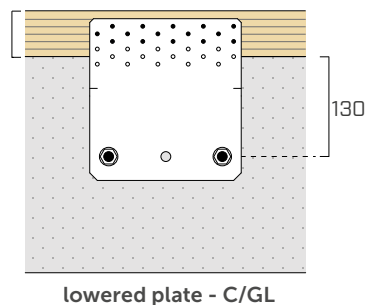
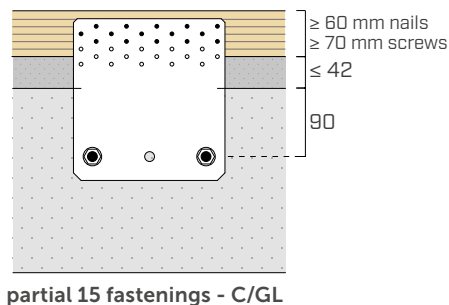
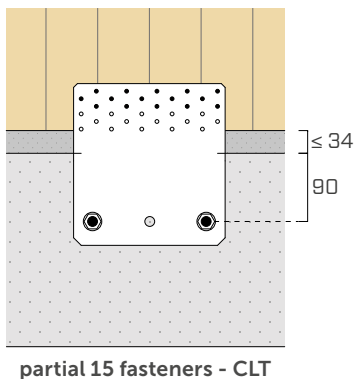
- C/GL: minimum distances for solid timber or glulam consistent with EN 1995:2014 according to ETA considering a timber density $\rho_k \leq 420 \text{ kg/m}^3$
- CLT minimum distances for Cross Laminated Timber according to ÖNORM EN 1995:2014 (Annex K) for nails and ETA-11/0030 for screws

FASTENING PATTERNS

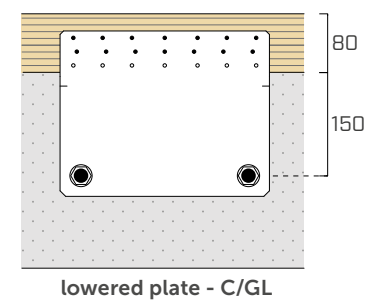
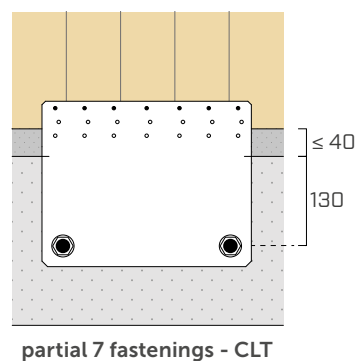
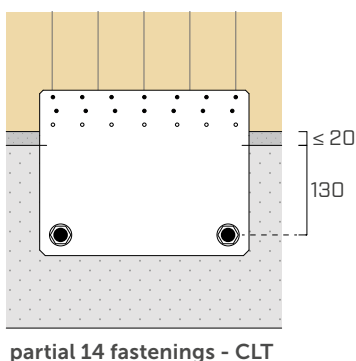
PARTIAL FASTENING

In the presence of design requirements such as varying stress values or the presence of a grout between the wall and the support surface, it is possible to use pre-calculated **partial nailing patterns** or to position the plates as required (e.g. lowered plates). Take care to respect the minimum distances indicated in the table and verify the strength of the anchor-to-concrete group taking into account the increase in distance from the edge (c_x). Below there are some examples of possible limit configurations:

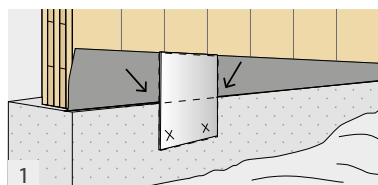
TCP200



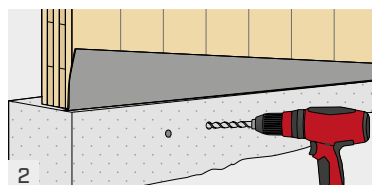
TCP300



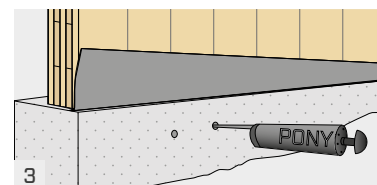
MOUNTING



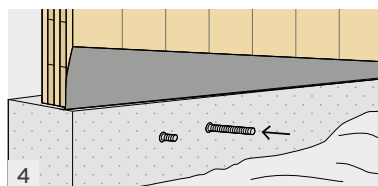
Positioning of the TITAN TCP with the dashed line at the timber-concrete interface and hole marking.



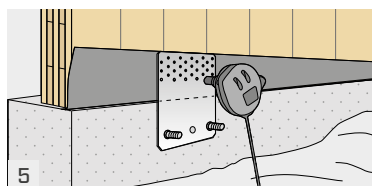
Removal of the TITAN TCP plate and drilling of the concrete support.



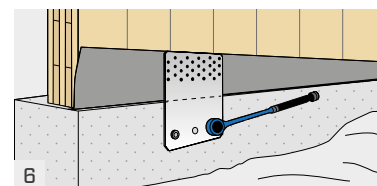
Accurate hole cleaning.



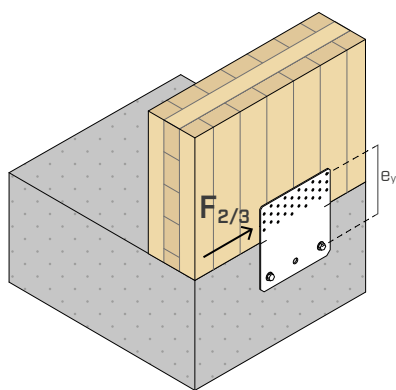
Injection of the anchor and insertion of the threaded rods into the holes.



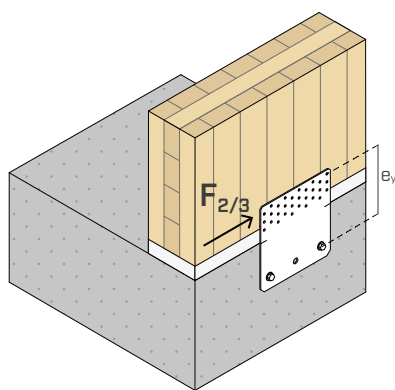
Installation of the TITAN TCP and nailing.



Positioning of nuts and washers by adequate tightening.



total fastening



partial fastening

TIMBER STRENGTH

	TIMBER					STEEL		CONCRETE				
configuration on timber	fastening holes Ø5			R _{2/3,k} timber ⁽¹⁾	R _{2/3,k} CLT ⁽²⁾	R _{2/3,k} steel		fastening holes Ø13				
	type	Ø x L [mm]	n _v [pcs]	[kN]	[kN]	[kN]	γ _{steel}	Ø [mm]	n _v [pcs]	e _y ⁽³⁾ [mm]		
total fastening	LBA	Ø4 x 60	30	62,9	84,9	21,8	γ _{M2}	M12	2	147		
	LBS	Ø5 x 60	30	54,0	69,8							
partial fastening	LBA	Ø4 x 60	15	31,5	42,5	20,5	γ _{M2}			M12	2	162
	LBS	Ø5 x 60	15	27,0	34,9							

CONCRETE STRENGTH

Concrete strength values of some of the possible anchoring solutions, according to the configurations adopted for fastening on timber (e_y). It is assumed that the plate is positioned with the assembly notches at the timber-to-concrete interface (distance between anchor and concrete edge $c_x = 90$ mm).

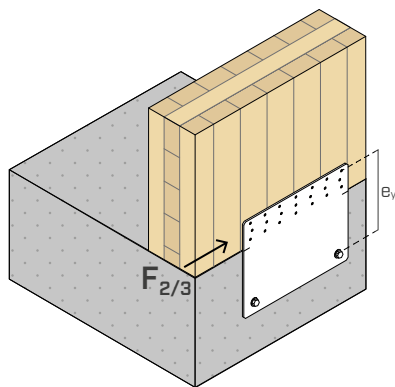
configuration on concrete	fastening holes Ø13		total fastening ($e_y = 147$ mm)	partial fastening ($e_y = 162$ mm)
	type	Ø x L [mm]	$R_{2/3,d}$ concrete	$R_{2/3,d}$ concrete
			[kN]	[kN]
uncracked	VIN-FIX 5.8	M12 x 140	12,6	11,5
		M12 x 195	13,4	12,2
	SKR	12 x 90	11,3	10,3
	AB1	M12 x 115	9,6	8,7
cracked	VIN-FIX 5.8	M12 x 140	8,9	8,1
		M12 x 195	9,5	8,7
	SKR	12 x 90	8,0	7,3
	AB1	M12 x 115	6,8	6,2
seismic	HYB-FIX 8.8	M12 x 140	6,6	6,1
		M12 x 195	8,1	7,4
	EPO-FIX 8.8	M12 x 140	7,6	6,9

NOTES

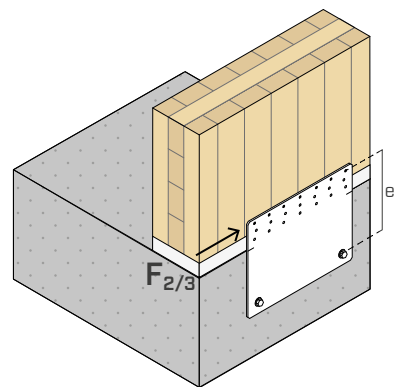
⁽¹⁾ Strength values for use on solid timber or glulam platform beam, calculated considering the effective number according to Table 8.1 (EN 1995:2014).

⁽²⁾ Strength values for use on CLT.

⁽³⁾ Eccentricity of calculation for verification of the anchor-to-concrete group.



total fastening



partial fastening

TIMBER STRENGTH

	TIMBER					STEEL		CONCRETE				
configuration on timber	fastening holes Ø5			R _{2/3,k} timber ⁽¹⁾	R _{2/3,k} CLT ⁽²⁾	R _{2/3,k} steel		fastening holes Ø17				
	type	Ø x L [mm]	n _v [pcs]	[kN]	[kN]	[kN]	γ _{steel}	Ø [mm]	n _v [pcs]	e _y ⁽³⁾ [mm]		
total fastening	LBA	Ø4 x 60	21	43,4	59,4	64,0	γ _{M2}	M16	2	180		
	LBS	Ø5 x 60	21	36,8	48,9							
partial fastening 14 fasteners	LBA	Ø4 x 60	14	29,0	39,6	60,5	γ _{M2}					190
	LBS	Ø5 x 60	14	24,6	32,6							
partial fastening 7 fasteners	LBA	Ø4 x 60	7	14,5	19,8	57,6	γ _{M2}					200
	LBS	Ø5 x 60	7	12,3	16,3							

CONCRETE STRENGTH

Concrete strength values of some of the possible anchoring solutions, according to the configurations adopted for fastening on timber (e_y). It is assumed that the plate is positioned with the assembly notches at the timber-to-concrete interface (distance between anchor and concrete edge $c_x = 130$ mm).

configuration on concrete	fastening holes Ø17		total fastening ($e_y = 180$ mm)	partial fastening ($e_y = 190$ mm)	partial fastening ($e_y = 200$ mm)
	type	Ø x L [mm]	$R_{2/3,d}$ concrete	$R_{2/3,d}$ concrete	$R_{2/3,d}$ concrete
			[kN]	[kN]	[kN]
uncracked	VIN-FIX 5.8	M16 x 195	29,6	28,3	27,0
	SKR	16 x 130	26,0	24,8	23,7
	AB1	M16 x 145	30,2	28,7	27,3
cracked	VIN-FIX 5.8	M16 x 195	21,0	20,0	19,1
	SKR	16 x 130	18,4	17,6	16,8
	AB1	M16 x 145	21,4	20,3	19,3
seismic	HYB-FIX 8.8	M16 x 195	16,8	16,2	15,6
		M16 x 245	18,6	17,7	16,9
	EPO-FIX 8.8	M16 x 195	17,8	17,0	16,9

GENERAL PRINCIPLES

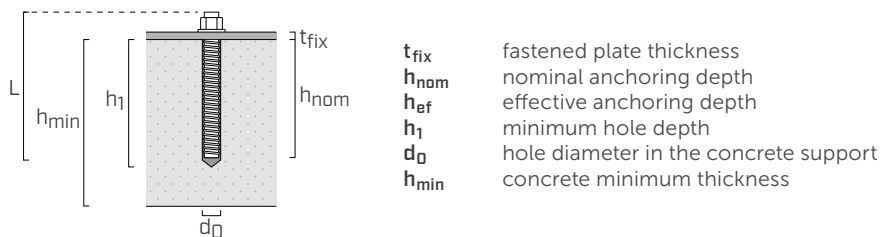
For the GENERAL PRINCIPLES of calculation, see page 8.

ANCHORS INSTALLATION PARAMETERS

installation	anchor type		t_{fix}	h_{ef}	h_{nom}	h_1	d_0	h_{min}
	type	$\varnothing \times L$ [mm]	[mm]	[mm]	[mm]	[mm]	[mm]	[mm]
TCP200	VIN-FIX 5.8 HYB-FIX 8.8 EPO-FIX 8.8	M12 x 140	3	112	112	120	14	150
	SKR	12 x 90	3	64	87	110	10	
	AB1	M12 x 115	3	70	85	105	12	
	VIN-FIX 5.8 HYB-FIX 8.8	M12 x 195	3	170	170	175	14	200
TCP300	VIN-FIX 5.8 HYB-FIX 8.8 EPO-FIX 8.8	M16 x 195	4	164	164	170	18	200
	SKR	16 x 130	4	85	126	150	14	
	AB1	M16 x 145	4	85	97	105	16	
	HYB-FIX 8.8	M16 x 245	4	210	210	215	18	250

Precut INA threaded rod, with nut and washer.

MGS threaded rod class 8.8 to be cut to size: For more information, see the "PLATES AND CONNECTORS FOR TIMBER" catalogue, available in the "Catalogues" section of the website www.rothoblaas.com



ANCHORS VERIFICATION FOR STRESS LOADING $F_{2/3}$

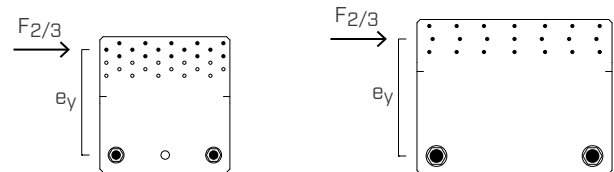
Fastening to concrete using anchors must be verified on the basis of the load acting on the anchors, which depend on the timber fastening configuration.

The position and number of nails/screws determine the ey eccentricity value, understood as the distance between the centre of gravity of the nailing and that of the anchors.

The anchor group must be verified for:

$$V_{Sd,x} = F_{2/3,d}$$

$$M_{Sd,z} = F_{2/3,d} \cdot e_y$$



GENERAL PRINCIPLES

- Characteristic values according to EN 1995:2014.
- Design values can be obtained from characteristic values as follows:

$$R_d = \min \left\{ \begin{array}{l} \frac{(R_{k, \text{timber}} \text{ or } R_{k, \text{CLT}}) \cdot k_{mod}}{\gamma_M} \\ \frac{R_{k, \text{steel}}}{\gamma_{M2}} \\ R_{d, \text{concrete}} \end{array} \right.$$

The coefficients k_{mod} , γ_M and γ_{M2} should be taken according to the current regulations used for the calculation.

- The calculation process used a timber characteristic density of $\rho_k = 350 \text{ kg/m}^3$ and C25/30 concrete with a thin reinforcing layer and minimum thickness indicated in the table.
- Dimensioning and verification of timber and concrete elements must be carried out separately.
- The strength values are valid for the calculation hypothesis defined in the table; for boundary conditions different from the ones in the table (e.g. minimum distances from the edge), the anchors-to-concrete can be verified using MyProject calculation software according to the design requirements.

- Seismic design in performance category C2, without ductility requirements on anchors (option a2) and elastic design according to EN 1992:2018. For chemical anchors it is assumed that the annular space between the anchor and the plate hole is filled ($\alpha_{gap} = 1$).
- The product ETAs for the anchors used in the concrete-side strength calculation are indicated below:
 - VIN-FIX chemical anchor according to ETA-20/0363;
 - HYB-FIX chemical anchor according to ETA-20/1285;
 - EPO-FIX chemical anchor according to ETA-23/0419;
 - SKR screw-in anchor according to ETA-24/0024;
 - AB1 mechanical anchor according to ETA-25/0860 (M12);
 - AB1 mechanical anchor according to ETA-99/0010 (M16).

INTELLECTUAL PROPERTY

- TITAN PLATE C plates are protected by the following Registered Community Designs:
 - RCD 002383265-0003;
 - RCD 008254353-0014.

■ EXPERIMENTAL INVESTIGATIONS | TCP300

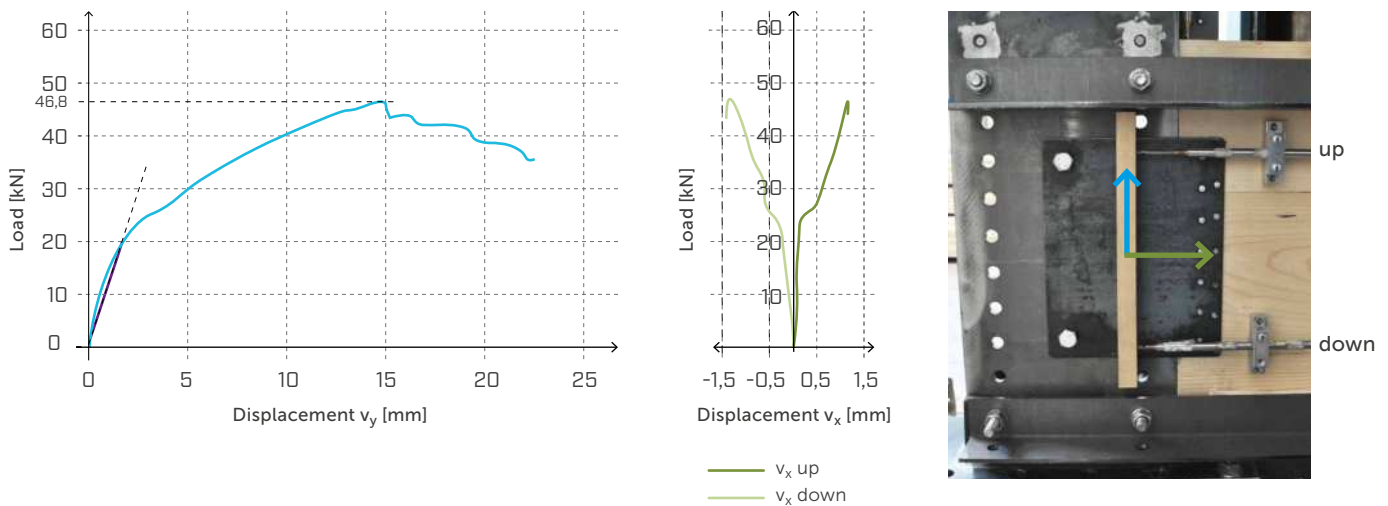
In order to calibrate the numerical models used for the design and verification of the TCP300 plate, an experimental campaign was carried out in collaboration with the Institute for BioEconomy (IBE) - San Michele all'Adige.

The connection system nailed or screwed to CLT panels has been shear stressed through monotonic tests in displacement control, registering the load, displacement in the two main directions and collapse mode.

The results obtained were used to validate the analytical calculation model for the TCP300 plate, based on the hypothesis that the shear centre is placed at the centre of gravity of the fastenings on timber. Therefore that the anchors, usually the weak point of the system, are stressed not only by the shear actions but also by the local moment.

The study in different fastening configurations (Ø4 nails/Ø5 screws, full nailing, partial nailing with 14 connectors, partial nailing with 7 connectors) shows that the mechanical behaviour of the plate is strongly influenced by the **relative stiffness of the connectors** on timber compared to that of the anchors, in tests simulated by bolting on steel.

In all cases a shear failure mode of the timber fasteners has been observed, which does not result in evident plate rotation. Only in some cases (full nailing) the non-negligible rotation of the plate leads to an increase in stress on the timber fasteners resulting from a redistribution of the local moment with consequent stress relief on the anchors, which represent the limiting point of the overall strength of the system.



Load-to-displacement diagrams for TCP300 specimen with partial nailing (no. 14 LBA Ø4 x 60 mm nails).

Further investigations are necessary in order to define an analytical model that can be generalized to the different configurations of use of the plate that is able to provide the actual stiffness of the system and the redistribution of stresses as the boundary conditions (connectors and base materials) vary.